



AMERICAN WOOD COUNCIL

March 2025

ERRATA
to the 2024 *National Design Specification (NDS) for Wood Construction*

(All prior PDF and print versions)

In 15.2.3.3, add “ K_x ” in the equation for F_{cE} as shown below.

$$F_{cE} = \frac{0.822 K_x E_{min}'}{(\ell_e/d)^2}$$



ERRATA
to the 2024 National Design Specification (NDS) for Wood Construction

(All prior PDF and print versions)

The term “10D” in the second equation for K_D in Table 12.3.1B is incorrectly depicted as “10D” and is corrected as shown below.

Table 12.3.1B – Reduction Term, R_d		
Fastener Size	Yield Mode	Reduction Term, R_d
$0.25" \leq D \leq 1"$	I_m, I_s	$4 K_\theta$
	II	$3.6 K_\theta$
	III_m, III_s, IV	$3.2 K_\theta$
$D < 0.25"$	$I_m, I_s, II, III_m, III_s, IV$	K_D^1

Notes:

$K_\theta = 1 + 0.25(\theta/90)$

θ = maximum angle between the direction of load and the direction of grain ($0^\circ \leq \theta \leq 90^\circ$) for any member in a connection

D = diameter, in. (see 12.3.7)

$K_D = 2.2$ for $D \leq 0.17"$

$K_D = 10D - 0.5$ for $0.17" < D < 0.25"$

¹ For threaded fasteners where nominal diameter (see Appendix L) is greater than or equal to 0.25" and root diameter is less than 0.25", $R_d = K_D K_\theta$.

should be 10D

The term “D^{1.5}” in the equations for γ in 11.3.6.1 is incorrectly depicted as “D_{1.5}” and is corrected as shown below.

11.3.6 Group Action Factors, C_g

11.3.6.1 Reference lateral design values for split ring connectors, shear plate connectors, or dowel-type fasteners with $D \leq 1"$ in a row shall be multiplied by the following group action factor, C_g:

$$C_g = \left[\frac{m(1-m^{2n})}{n[(1+R_{EA}m^n)(1+m)-1+m^{2n}]} \right] \left[\frac{1+R_{EA}}{1-m} \right] \quad (\text{Eq. 11.3-1})$$

where:

C_g = 1.0 for dowel type fasteners with $D < 1/4"$

N = number of fasteners in a row

R_{EA} = the lesser of $\frac{E_s A_s}{E_m A_m}$ or $\frac{E_m A_m}{E_s A_s}$

E_m = modulus of elasticity of main member, psi

E_s = modulus of elasticity of side members, psi

A_m = gross cross-sectional area of main member, in.²

A_s = sum of gross cross-sectional areas of side members, in.²

$m = u - \sqrt{u^2 - 1}$

$u = 1 + \gamma \frac{s}{2} \left[\frac{1}{E_m A_m} + \frac{1}{E_s A_s} \right]$

s = center to center spacing between adjacent fasteners in a row, in.

γ = load/slip modulus for a connection, lbs/in.

= 500,000 lbs/in. for 4" split ring or shear plate connectors

= 400,000 lbs/in. for 2-1/2" split ring or 2-5/8" shear plate connectors

= (180,000) D_{1.5} for dowel-type fasteners in wood-to-wood connections

= (270,000) D_{1.5} for dowel-type fasteners in wood-to-metal connections

D = diameter of dowel-type fastener, in.

should be D^{1.5}



ERRATA
to the **2024 National Design Specification (NDS) for Wood Construction**

(All prior PDF and print versions)

Table 14.2.2A is revised as shown below in red underline. Prior values were based on $S_q=1-1/4"$ rather than $S_q=1-1/2"$.

Table 14.2.2A Values of q_w (lbs) Perpendicular to Grain for Timber Rivets

$s_p = 1"$

s_q in.	Rivets per row	Number of rows				
		2	4	6	8	10
1	2	776	809	927	1089	1255
	3	768	806	910	1056	1202
	4	821	870	963	1098	1232
	5	874	923	1013	1147	1284
	6	959	1007	1094	1228	1371
	7	1048	1082	1163	1297	1436
	8	1173	1184	1256	1391	1525
	9	1237	1277	1345	1467	1624
	10	1318	1397	1460	1563	1752
	11	1420	1486	1536	1663	1850
	12	1548	1597	1628	1786	1970
	13	1711	1690	1741	1882	2062
	1-1/2	2	<u>1136</u>	<u>1097</u>	<u>1221</u>	<u>1414</u>
3		<u>1124</u>	<u>1093</u>	<u>1199</u>	<u>1371</u>	<u>1561</u>
4		<u>1202</u>	<u>1180</u>	<u>1268</u>	<u>1426</u>	<u>1601</u>
5		<u>1280</u>	<u>1251</u>	<u>1334</u>	<u>1490</u>	<u>1668</u>
6		<u>1404</u>	<u>1366</u>	<u>1442</u>	<u>1595</u>	<u>1780</u>
7		<u>1534</u>	<u>1467</u>	<u>1532</u>	<u>1685</u>	<u>1865</u>
8		<u>1717</u>	<u>1606</u>	<u>1654</u>	<u>1806</u>	<u>1980</u>
9		1811	1731	1772	1905	2110

Figure 5F is revised as shown on the following page - to be consistent with NDS provisions for compression side notching of structural glued-laminated timber in 5.4.5.2, which states that “Compression side end-notches shall not extend into the middle 1/3 of the span nor more than $3d_n$ from the end of the member.”

Figure 5F – Compression Face Notch and Taper Cut Limitations for Glulam Beams

