## ERRATA

**ERRATA**

to the 2018 and Prior Editions of

*the National Design Specification® (NDS®) for Wood Construction*

<table>
<thead>
<tr>
<th>Page</th>
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<tbody>
<tr>
<td>91</td>
<td>Revise footnote 1 in Table 12.5.1D as follows:</td>
</tr>
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</table>

1. The $\ell/D$ ratio used to determine the minimum edge distance spacing between rows shall be the lesser of:
   - (a) length of fastener in wood main member/D = $\ell_m/D$
   - (b) total length of fastener in wood side member(s)/D = $\ell_s/D$
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Page 166

Clarifies that the following calculations in Example E.7 Sample Solution of Row of Bolts is intended for a single-row bolted connection with a 3-1/2" thick main member and 1-1/2" thick side member:

E.7 Sample Solution of Row of Bolts

Calculate the net section area tension and row tear-out adjusted ASD design capacities for the single-shear single-row bolted connection represented in Figure E2.

Main and Side Members:
#2 grade Hem-Fir 2x4 lumber. See NDS Supplement Table 4A – Visually Graded Dimension Lumber for reference design values. Adjustment factors C_D, C_T, C_M, and C_i are assumed to equal 1.0 in this example for calculation of adjusted design values.

F_t' = 525 psi (C_T) = 525(1.5) = 788 psi
F_v' = 150 psi

Connection Details:
Bolt diameter, D: 1/2 in.
Bolt hole diameter, D_h: 0.5625 in.
Adjusted ASD bolt design value, Z_{||}': 550 lbs
(See NDS Table 12A for 3-1/2” main member thickness and 1-1/2” side member thickness. For this trial design, the group action factor, C_G, is taken as 1.0).

Adjusted ASD Connection Capacity, n Z_{||}':

nZ_{||}' = (3 bolts)(550 lbs) = 1,650 lbs

Adjusted For side member, adjusted ASD Net Section Area Tension Capacity, Z_{NT}':

Z_{NT}' = F_t' [w - n_{row} D_h]
Z_{NT}' = (788 psi)[(3.5" - 1(0.5625"))] = 3,470 lbs

Adjusted For side member, adjusted ASD Row Tear-Out Capacity, Z_{RT}':

Z_{RT}' = n F_v' t_{critical}
Z_{RT}' = 3(150 psi)(1.5") = 1,350 lbs

In this sample calculation, the adjusted ASD connection capacity is limited to 1,350 pounds by row tear-out, Z_{RT}'.

Figure E2 Single Row of Bolts
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to the 2012 Edition of
the National Design Specification® (NDS®) for Wood Construction

Page  Revision
40    Revise $K_{rs}$ as described in Equation (5.4-3) as follows (replace $d_e$ with $d_c$):

\[
K_{rs} = \text{empirical radial stress factor} \\
= 0.29 \left( \frac{d_c d_{cr}}{R_m} \right) + 0.32 \tan^{1.2} \phi_T
\]
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to the 2012 Edition of

*the National Design Specification® (NDS®) for Wood Construction*

(web versions dated 11-11 and 10-12, printed version 10-12)

**Page 38**
Revise Equations 5.3-4 and 5.3-5 as follows:

\[
C_i = \frac{1}{\sqrt{1 + \left(\frac{F_{b} \tan \theta}{F_{v} c_{PP}}\right)^2 + \left(\frac{F_{h} \tan^2 \theta}{F_{c+}}\right)^2}} \quad (5.3-4)
\]

\[
C_i = \frac{1}{\sqrt{1 + \left(\frac{F_{b} \tan \theta}{F_{v} c_{PP}}\right)^2 + \left(\frac{F_{h} \tan^2 \theta}{F_{c+}}\right)^2 \left(\frac{F_{c+}}{F_{c+}\theta}\right)^2}} \quad (5.3-5)
\]

**Page 82**
Revise the last sentences in Section 11.3.5.2 as follows:

“Where \(p\) includes the length of a tapered tip, \(E\), the dowel bearing length, \(\ell_{s,i}\) or \(\ell_{m,i}\), shall not exceed \(p - E/2\).

a) For Lag screws, \(E_i\) is permitted to be taken from Appendix L, Table L2.

b) For wood screws, nails, and spikes, \(E_i\) is permitted to be taken as \(2D\).
### ERRATA

to the 2011 Edition of

*the National Design Specification® (NDS®) for Wood Construction*

(web version dated 11-11)

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<tr>
<td>75</td>
<td>11.2.3.4 11.2.3.5</td>
<td>Nails and spikes shall not be loaded in withdrawal from end grain of wood.</td>
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</table>