AMERICAN WOOD COUNCIL

August 2026

ERRATA

to the 2018 *National Design Specification (NDS) for Wood Construction*

(All prior PDF and print versions)

Correct the following table reference in 12.1.3.3 as shown in red strike-out and underline below.

12.1.3.3 A standard cut washer (Appendix Table ~~L6~~ L7), or metal plate or metal strap of equal or greater dimensions shall be provided between the wood and the bolt head and between the wood and the nut.

AMERICAN WOOD COUNCIL

December 2024

ERRATA

to the 2018 Edition of the

National Design Specification (NDS) for Wood Construction

(All prior PDF and print versions)

Page **Revision**

143 Table 14.2.2A is revised as shown below in red underline. Prior values were based on s_q=1-1/4" rather than s_q=1-1/2".

Table 14.2.2A Values of q_w (lbs) Perpendicular to Grain for Timber Rivets

$s_p = 1"$

s_q in.	Rivets per row	Number of rows				
		2	4	6	8	10
1	2	776	809	927	1089	1255
	3	768	806	910	1056	1202
	4	821	870	963	1098	1232
	5	874	923	1013	1147	1284
	6	959	1007	1094	1228	1371
	7	1048	1082	1163	1297	1436
	8	1173	1184	1256	1391	1525
	9	1237	1277	1345	1467	1624
	10	1318	1397	1460	1563	1752
	11	1420	1486	1536	1663	1850
	12	1548	1597	1628	1786	1970
	13	1711	1690	1741	1882	2062
1-1/2	2	1136	1097	1221	1414	1630
	3	1124	1093	1199	1371	1561
	4	1202	1180	1268	1426	1601
	5	1280	1251	1334	1490	1668
	6	1404	1366	1442	1595	1780
	7	1534	1467	1532	1685	1865
	8	1717	1606	1654	1806	1980
	9	1811	1731	1772	1905	2110

AMERICAN WOOD COUNCIL

July 2024

ERRATA

to the 2018, 2015, and 2012 Editions of Commentary for the *National Design Specification (NDS) for Wood Construction*

(All prior PDF and print versions)

Page	Revision
214	Revise equation C4.2.4-1 as shown in red below:

$$E_{min} = \frac{\mathrm{E}(1\text{-}1.645COV_E)(1.03)}{1.66} \qquad \text{(C4.2.4-1)}$$

AMERICAN WOOD COUNCIL

January 2024

ERRATA

to the 2018 Edition of Commentary for the *National Design Specification (NDS) for Wood Construction*

(All prior PDF and print versions)

Page	Revision
256	Revise equation C12.2.2-2 as shown in red below:

$$K_W = 1.2\left(\frac{14250}{6}\right) \qquad \text{(C12.2.2-2)}$$

AMERICAN WOOD COUNCIL

January 2021

ERRATA
to the 2018 and Prior Editions of
the National Design Specification® (NDS®) for Wood Construction

Page	Revision
91	Revise footnote 1 in Table 12.5.1D as follows:

1. The ℓ/D ratio used to determine the minimum ~~edge distance~~ spacing between rows shall be the lesser of:
   (a) length of fastener in wood main member/D = ℓ_m/D
   (b) total length of fastener in wood side member(s)/D = ℓ_s /D

AMERICAN WOOD COUNCIL

July 2020

ERRATA
to the 2018 and Prior Editions of
the National Design Specification® (NDS®) for Wood Construction

Page	Revision
166	Clarifies that the following calculations in Example E.7 Sample Solution of Row of Bolts is intended for a single-row bolted connection with a 3-1/2″ thick main member and 1-1/2″ thick side member:

E.7 Sample Solution of Row of Bolts

Calculate the net section area tension and row tear-out adjusted ASD design capacities for the single-shear single-row bolted connection represented in Figure E2.

Main and Side Members:
#2 grade Hem-Fir ~~2x4~~ lumber. See *NDS Supplement* Table 4A – Visually Graded Dimension Lumber for reference design values. Adjustment factors C_D, C_T, C_M, and C_i are assumed to equal 1.0 in this example for calculation of adjusted design values.

$F_t' = 525 \text{ psi } (C_F) = 525(1.5) = 788 \text{ psi}$

$F_v' = 150 \text{ psi}$

Connection Details:
Bolt diameter, D: 1/2 in.
Bolt hole diameter, D_h: 0.5625 in.
Adjusted ASD bolt design value, $Z_{\|}'$: 550 lbs
(See NDS Table 12A for 3-1/2″ main member thickness and 1-1/2″ side member thickness. For this trial design, the group action factor, C_g, is taken as 1.0).

Adjusted ASD Connection Capacity, n $Z_{\|}'$:

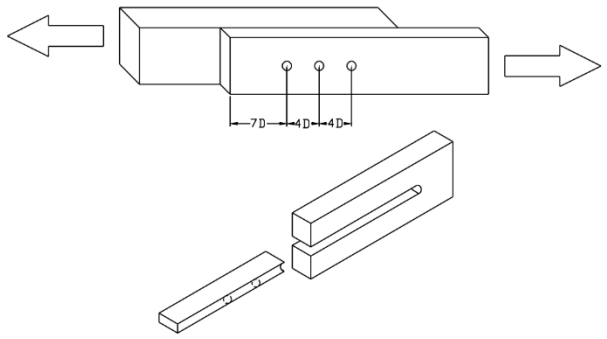
$nZ_{\|}' = (3 \text{ bolts})(550 \text{ lbs}) = 1{,}650 \text{ lbs}$

~~Adjusted~~ For side member, adjusted ASD Net Section Area Tension Capacity, Z_{NT}':

$$Z_{NT}' = F_t' \, t \, [w - n_{row} \, D_h]$$

$Z_{NT}' = (788 \text{ psi})(1.5'')[3.5'' - 1(0.5625'')] = 3{,}470 \text{ lbs}$

Figure E2 Single Row of Bolts



~~Adjusted~~ For side member, adjusted ASD Row Tear-Out Capacity, Z_{RT}':

$$Z_{RTi}' = n_i F_v' \, ts_{critical}$$

$Z_{RT1}' = 3(150 \text{ psi})(1.5'')(2'') = 1{,}350 \text{ lbs}$

In this sample calculation, the adjusted ASD connection capacity is limited to 1,350 pounds by row tear-out, Z_{RT}'.

AMERICAN WOOD COUNCIL

March 2019

ERRATA
to the 2018 Edition of
the National Design Specification® (NDS®) for Wood Construction
(all versions)

Page	Revision
167	Revise the following calculations in Example E.8 Sample Solution of Row of Split Rings (remainder of example is unchanged):

E.8 Sample Solution of Row of Split Rings

Calculate the net section area tension and row tear-out adjusted ASD design capacities for the single-shear single- row split ring connection represented in Figure E3.

Main and Side Members:
#2 grade Southern Pine 2x4 lumber. See *NDS Supplement* Table 4B – Visually Graded Southern Pine Dimension Lumber for reference design values. Adjustment factors C_D, C_T, C_M, and C_i are assumed to equal 1.0 in this example for calculation of adjusted design values.

$F_t' = \cancel{825}\ 675$ psi

$F_v' = 175$ psi

Main member thickness, t_m: 1.5 in.
Side member thickness, t_s: 1.5 in.
Main and side member width, w: 3.5 in.

Connection Details:
Split ring diameter, D: 2.5 in. (see Appendix K for connector dimensions)
Adjusted ASD split ring design value, P′: 2,730 lbs (see Table 13.2A. For this trial design, the group action factor, C_g, is taken as 1.0).

Adjusted ASD Connection Capacity, nP′:

$nP' = (2 \text{ split rings})(2{,}730 \text{ lbs}) = 5{,}460 \text{ lbs}$

Adjusted ASD Net Section Area Tension Capacity, Z_{NT}':

$$Z_{NT}' = F_t' A_{net}$$

$Z_{NT}' = F_t' [A_{2x4} - A_{bolt\text{-}hole} - A_{split\ ring\ projected\ area}]$

$Z_{NT}' = (\cancel{825}\ 675 \text{ psi})[5.25 \text{ in.}^2 - 1.5\text{" (0.5625")} - 1.1 \text{ in.}^2]$

$= \cancel{2{,}728}\ 2{,}232$ lbs

Adjusted ASD Row Tear-Out Capacity, Z_{RT}':

$$Z_{RTi}' = n_i \frac{F_v' A_{critical}}{2}$$

$Z_{RT1}' = [(2 \text{ connectors})(175 \text{ psi})/2](21.735 \text{ in.}^2)$

$= 3{,}804$ lbs

where:

$A_{critical} = 21.735 \text{ in.}^2$ (See Figures E4 and E5)

In this sample calculation, the adjusted ASD connection capacity is limited to $\cancel{2{,}728}\ 2{,}232$ pounds by net section area tension capacity, Z_{NT}'.

AMERICAN WOOD COUNCIL

May 2018

ERRATA
to the 2018 Edition of
the National Design Specification® (NDS®) for Wood Construction
(web version dated 11-17)

Page	**Revision**
40	Revise K_{rs} as described in Equation (5.4-3) as follows (replace d_e with d_c):

K_{rs} = empirical radial stress factor

= $0.29(\cancel{d_e} d_c/R_m) + 0.32 \tan^{1.2} \phi_T$