**AMERICAN FOREST & PAPER ASSOCIATION**
American Wood Council
*Engineered and Traditional Wood Products*

June 2005

**2005 ERRATA**
**to the**

**2001 Edition of**

***the Allowable Stress Design (ASD) Manual for Engineered Wood Construction***
(printed version dated 06-02 20M)

| **Page** | **Revision** |
|---|---|
| 93 | In Figure 10.28, the equation for $R_2$ should be revised as follows: |

$$R_2 = V_2 + V_3 \ldots\ldots\ldots = \frac{Pa}{2\ell^3}\left(2\ell^2 + b\left(\ell + a\right)\right)$$

Change the "minus" sign to a "plus" sign in front of the variable "b."

**2005 ERRATA**
**to the**

**2001 Edition of**

***NATIONAL DESIGN SPECIFICATION® (NDS®) FOR WOOD CONSTRUCTION***
***included in the Allowable Stress Design (ASD) Manual for Engineered Wood Construction***
(printed versions dated 03-02 2M, 06-02 20M, 8-04 1M)

| **Page** | **Revision** |
|---|---|
| 112 | In section 12.3.6.1, subsections (b), (c), (d), and (e), revise each sentence to read: |

"Sloping surface with ~~a~~ $\alpha$ from 45° …"

Remainder of each sentence remains the same.

**AMERICAN FOREST & PAPER ASSOCIATION**
American Wood Council
*Engineered and Traditional Wood Products*

January 2005

**2005 ERRATA**
**to the**

**2001 Edition of**

***the Allowable Stress Design (ASD) Manual for Engineered Wood Construction***
(printed version dated 06-02 20M)

| **Page** | **Revision** |
|---|---|
| 32 | In Example 3-2, the text should be revised as follows: |

"Same as Example 3-1, but the chord includes connections with one row of ~~3/4~~ 7/8 inch bolts (in a 1/16 inch oversized hole)…"

All other text unchanged.

AMERICAN FOREST & PAPER ASSOCIATION
American Wood Council
*Engineered and Traditional Wood Products*

December 2004

**2004 ERRATA**
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**2001 Edition of**

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(printed versions dated 03-02 2M, 06-02 20M, 8-04 1M)

**Page** **Revision**

59 In **Table 10.3.3 Wet Service Factors, $C_M$, for Connections**, change the following:

| Fastener Type | Moisture Content | | $C_M$ |
|---|---|---|---|
| | At Time of Fabrication | In-Service | |
| Lateral Loads | | | |
| Metal Connector Plates[2] | any | > 19% | ~~0.7~~ 0.8 |

108 In **Table 12.2.3 Penetration Depth Factors, $C_d$, for Split Ring and Shear Plate Connectors Used with Lag Screws**, change "Species Group (see Table 10A)" to "Species Group (see Table 12A)"

149 In Section D.3, equation D-3 change "$F_{cE}$"to "$F_{bE}$".

**AMERICAN FOREST & PAPER ASSOCIATION**
American Wood Council
*Engineered and Traditional Wood Products*

May 2004

**2004 ADDENDUM**
**to the**

**2001 Edition of**

***SUPPLEMENT TO THE NDS® - DESIGN VALUES FOR WOOD CONSTRUCTION***
***included in the Allowable Stress Design (ASD) Manual for Engineered Wood Construction***
(printed version dated 06-02 20M)

**Page** **Revision**

4 In Table 2.1, add the following:

| Species or Species Combination | Species That May Be Included in Combination | Grading Rules Agencies | Design Values Provided in Tables |
|---|---|---|---|
| Alaska Cedar | Alaska Cedar | WCLIB | 4A |
| Alaska Hemlock | Alaska Hemlock | WWPA | 4A |
| Alaska Yellow Cedar | Alaska Yellow Cedar | WCLIB, WWPA | 4A |

31 In Table 4A, add the following species and design values:

| Species and commercial grade | Size classification | Design values in pounds per square inch (psi) | | | | | | |
|---|---|---|---|---|---|---|---|---|
| | | Bending $F_b$ | Tension parallel to grain $F_t$ | Shear parallel to grain $F_v$ | Compression perpendicular to grain $F_{c\perp}$ | Compression parallel to grain $F_c$ | Modulus of Elasticity E | Grading Rules Agency |
| **Alaska Cedar** | | | | | | | | |
| Select Structural | | 1,150 | 625 | 165 | 525 | 1,000 | 1,400,000 | |
| No. 1 | | 975 | 525 | 165 | 525 | 900 | 1,300,000 | |
| No. 2 | 2” & wider | 800 | 425 | 165 | 525 | 750 | 1,200,000 | |
| No. 3 | | 450 | 250 | 165 | 525 | 425 | 1,100,000 | WCLIB |
| Stud | 2” & wider | 625 | 350 | 165 | 525 | 475 | 1,100,000 | |
| Construction | | 900 | 500 | 165 | 525 | 950 | 1,200,000 | |
| Standard | 2”-4” wide | 500 | 275 | 165 | 525 | 775 | 1,100,000 | |
| Utility | | 250 | 125 | 165 | 525 | 500 | 1,000,000 | |
| **Alaska Hemlock** | | | | | | | | |
| Select Structural | | 1300 | 825 | 185 | 440 | 1200 | 1,700,000 | |
| No. 1 | | 900 | 550 | 185 | 440 | 1100 | 1,600,000 | |
| No. 2 | 2” & wider | 825 | 475 | 185 | 440 | 1050 | 1,500,000 | |
| No. 3 | | 475 | 275 | 185 | 440 | 600 | 1,400,000 | |
| Stud | 2” & wider | 650 | 375 | 185 | 440 | 650 | 1,400,000 | WWPA |
| Construction | | 950 | 550 | 185 | 440 | 1250 | 1,400,000 | |
| Standard | 2”-4” wide | 525 | 300 | 185 | 440 | 1050 | 1,300,000 | |
| Utility | | 250 | 150 | 185 | 440 | 700 | 1,200,000 | |
| **Alaska Yellow Cedar** | | | | | | | | |
| Select Structural | | 1350 | 800 | 225 | 510 | 1200 | 1,500,000 | |
| No. 1 | | 900 | 525 | 225 | 510 | 1050 | 1,400,000 | |
| No. 2 | 2” & wider | 800 | 450 | 225 | 510 | 1000 | 1,300,000 | |
| No. 3 | | 475 | 250 | 225 | 510 | 575 | 1,200,000 | WWPA |
| Stud | 2” & wider | 625 | 350 | 225 | 510 | 625 | 1,200,000 | WCLIB |
| Construction | | 925 | 500 | 225 | 510 | 1250 | 1,300,000 | |
| Standard | 2”-4” wide | 500 | 275 | 225 | 510 | 1050 | 1,100,000 | |
| Utility | | 250 | 125 | 225 | 510 | 675 | 1,100,000 | |

**Page** **Revision**

42 In footnote 2, add values for Douglas Fir-Larch (N) as follows:

| Species | Modulus of Elasticity E (x10$^6$) (psi) | Specific Gravity G | Shear Parallel to Grain $F_v$ (psi) | Compression Perpendicular to Grain $F_{c\perp}$ (psi) | Grading Rules Agency |
|---|---|---|---|---|---|
| Douglas Fir-Larch (N) | 1.0 and higher | 0.49 | 180 | 625 | NLGA |
| | 1.2 – 1.9 | 0.49 | 180 | 625 | |
| | 2.0 – 2.2 | 0.53 | 180 | 715 | |
| | 2.3 and higher | 0.57 | 190 | 715 | |

44 In Table 4D, add the following species and design values:

| Species and commercial grade | Size classification | Design values in pounds per square inch (psi) | | | | | | |
|---|---|---|---|---|---|---|---|---|
| | | Bending $F_b$ | Tension parallel to grain $F_t$ | Shear parallel to grain $F_v$ | Compression perpendicular to grain $F_{c\perp}$ | Compression parallel to grain $F_c$ | Modulus of Elasticity E | Grading Rules Agency |
| **Alaska Cedar** | | | | | | | | |
| Select Structural | Beams and Stringers | 1,400 | 675 | 155 | 525 | 925 | 1,200,000 | WCLIB |
| No. 1 | | 1,150 | 475 | 155 | 525 | 775 | 1,200,000 | |
| No. 2 | | 750 | 300 | 155 | 525 | 500 | 1,000,000 | |
| Select Structural | Posts and Timbers | 1,300 | 700 | 155 | 525 | 975 | 1,200,000 | |
| No. 1 | | 1,050 | 575 | 155 | 525 | 850 | 1,200,000 | |
| No. 2 | | 625 | 350 | 155 | 525 | 600 | 1,000,000 | |

**AMERICAN FOREST & PAPER ASSOCIATION**
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April 2003

# 2003 ERRATA
# to the

## *ASD STRUCTURAL LUMBER SUPPLEMENT*
## *included in the Allowable Stress Design (ASD) Manual for Engineered Wood Construction*
(printed version dated 06-02 20M)

**Page** **Revision**

L-55 In Table 7.10, replace Southern Pine Grade #2 and #3 Maximum Floor Joist Spans with:

| | | | Dead Load = 10 psf | | | | Dead Load = 20 psf | | | |
|---|---|---|---|---|---|---|---|---|---|---|
| | | | 2x6 | 2x8 | 2x10 | 2x12 | 2x6 | 2x8 | 2x10 | 2x12 |
| | | | Maximum Floor Joist Spans | | | | | | | |
| Joist Spacing | Species and Grade | | (ft.-in.) | (ft.-in.) | (ft.-in.) | (ft.-in.) | (ft.-in.) | (ft.-in.) | (ft.-in.) | (ft.-in.) |
| 12 in. | Southern Pine | #2 | 10 - 9 | 14 - 2 | 18 - 0 | 21 - 9 | 10 - 9 | 14 - 2 | ~~17 - 9~~ 16 - 11 | 19 - 10 |
| | Southern Pine | #3 | ~~10 - 3~~ 9 - 4 | ~~13 - 0~~ 11 - 11 | ~~14 - 9~~ 14 - 0 | 16 - 8 | ~~9 - 8~~ 8 - 6 | ~~11 - 11~~ 10 - 10 | ~~13 - 5~~ 12 - 10 | 15 - 3 |
| 16 in. | Southern Pine | #2 | 9 - 9 | 12 - 10 | ~~16 - 5~~ 16 - 1 | 18 - 10 | ~~9 - 9~~ 9 - 6 | ~~12 - 10~~ 12 - 4 | ~~15 - 5~~ 14 - 8 | 17 - 2 |
| | Southern Pine | #3 | ~~9 - 3~~ 8 - 1 | ~~11 - 3~~ 10 - 3 | ~~12 - 9~~ 12 - 2 | 14 - 6 | ~~8 - 5~~ 7 - 4 | ~~10 - 3~~ 9 - 5 | ~~11 - 8~~ 11 - 1 | 13 - 2 |
| 19.2 in. | Southern Pine | #2 | 9 - 2 | 12 - 1 | ~~15 - 5~~ 14 - 8 | 17 - 2 | ~~9 - 2~~ 8 - 8 | ~~12 - 1~~ 11 - 3 | ~~14 - 1~~ 13 - 5 | 15 - 8 |
| | Southern Pine | #3 | ~~8 - 5~~ 7 - 4 | ~~10 - 3~~ 9 - 5 | ~~11 - 8~~ 11 - 1 | 13 - 2 | ~~7 - 8~~ 6 - 9 | ~~9 - 5~~ 8 - 7 | ~~10 - 7~~ 10 - 1 | 12 - 1 |
| 24 in. | Southern Pine | #2 | 8 - 6 | ~~11 - 3~~ 11 - 0 | ~~13 - 9~~ 13 - 1 | 15 - 5 | ~~8 - 6~~ 7 - 9 | ~~11 - 0~~ 10 - 0 | ~~12 - 7~~ 12 - 0 | 14 - 0 |
| | Southern Pine | #3 | ~~7 - 6~~ 6 - 7 | ~~9 - 2~~ 8 - 5 | ~~10 - 5~~ 9 - 11 | 11 - 10 | ~~6 - 10~~ 6 - 0 | ~~8 - 5~~ 7 - 8 | ~~9 - 6~~ 9 - 1 | 10 - 9 |

Alternatively, cut and paste the following onto page L-55 of the *ASD Lumber Supplement* to replace Table 7.10. If you would like to receive a self-adhesive version of the following page that can be permanently inserted in your document, please contact AWC with your mailing address.

## Table 7.10 Floor Joist Spans for Common Lumber Species

(Residential Areas, Live Load = 40 psf, L/Δ = 360)

| | | | Dead Load = 10 psf | | | | Dead Load = 20 psf | | | |
|---|---|---|---|---|---|---|---|---|---|---|
| | | | 2x6 | 2x8 | 2x10 | 2x12 | 2x6 | 2x8 | 2x10 | 2x12 |
| | | | Maximum Floor Joist Spans | | | | | | | |
| Joist Spacing | Specie and Grade | | (ft.-in.) | (ft.-in.) | (ft.-in.) | (ft.-in.) | (ft.-in.) | (ft.-in.) | (ft.-in.) | (ft.-in.) |
| 12 in. | Douglas Fir-Larch | SS | 11 - 4 | 15 - 0 | 19 - 1 | 23 - 3 | 11 - 4 | 15 - 0 | 19 - 1 | 23 - 3 |
| | Douglas Fir-Larch | #1 | 10 - 11 | 14 - 5 | 18 - 5 | 22 - 0 | 10 - 11 | 14 - 2 | 17 - 4 | 20 - 1 |
| | Douglas Fir-Larch | #2 | 10 - 9 | 14 - 2 | 18 - 0 | 20 - 11 | 10 - 8 | 13 - 6 | 16 - 5 | 19 - 1 |
| | Douglas Fir-Larch | #3 | 8 - 11 | 11 - 3 | 13 - 9 | 16 - 0 | 8 - 1 | 10 - 3 | 12 - 7 | 14 - 7 |
| | Hem-Fir | SS | 10 - 9 | 14 - 2 | 18 - 0 | 21 - 11 | 10 - 9 | 14 - 2 | 18 - 0 | 21 - 11 |
| | Hem-Fir | #1 | 10 - 6 | 13 - 10 | 17 - 8 | 21 - 6 | 10 - 6 | 13 - 10 | 17 - 1 | 19 - 10 |
| | Hem-Fir | #2 | 10 - 0 | 13 - 2 | 16 - 10 | 20 - 4 | 10 - 0 | 13 - 1 | 16 - 0 | 18 - 6 |
| | Hem-Fir | #3 | 8 - 8 | 11 - 0 | 13 - 5 | 15 - 7 | 7 - 11 | 10 - 0 | 12 - 3 | 14 - 3 |
| | Southern Pine | SS | 11 - 2 | 14 - 8 | 18 - 9 | 22 - 10 | 11 - 2 | 14 - 8 | 18 - 9 | 22 - 10 |
| | Southern Pine | #1 | 10 - 11 | 14 - 5 | 18 - 5 | 22 - 5 | 10 - 11 | 14 - 5 | 18 - 5 | 22 - 5 |
| | Southern Pine | #2 | 10 - 9 | 14 - 2 | 18 - 0 | 21 - 9 | 10 - 9 | 14 - 2 | 16 - 11 | 19 - 10 |
| | Southern Pine | #3 | 9 - 4 | 11 - 11 | 14 - 0 | 16 - 8 | 8 - 6 | 10 - 10 | 12 - 10 | 15 - 3 |
| | Spruce-Pine Fir | SS | 10 - 6 | 13 - 10 | 17 - 8 | 21 - 6 | 10 - 6 | 13 - 10 | 17 - 8 | 21 - 6 |
| | Spruce-Pine Fir | #1 | 10 - 3 | 13 - 6 | 17 - 3 | 20 - 7 | 10 - 3 | 13 - 3 | 16 - 3 | 18 - 10 |
| | Spruce-Pine Fir | #2 | 10 - 3 | 13 - 6 | 17 - 3 | 20 - 7 | 10 - 3 | 13 - 3 | 16 - 3 | 18 - 10 |
| | Spruce-Pine Fir | #3 | 8 - 8 | 11 - 0 | 13 - 5 | 15 - 7 | 7 - 11 | 10 - 0 | 12 - 3 | 14 - 3 |
| 16 in. | Douglas Fir-Larch | SS | 10 - 4 | 13 - 7 | 17 - 4 | 21 - 1 | 10 - 4 | 13 - 7 | 17 - 4 | 21 - 1 |
| | Douglas Fir-Larch | #1 | 9 - 11 | 13 - 1 | 16 - 5 | 19 - 1 | 9 - 8 | 12 - 4 | 15 - 0 | 17 - 5 |
| | Douglas Fir-Larch | #2 | 9 - 9 | 12 - 9 | 15 - 7 | 18 - 1 | 9 - 3 | 11 - 8 | 14 - 3 | 16 - 6 |
| | Douglas Fir-Larch | #3 | 7 - 8 | 9 - 9 | 11 - 11 | 13 - 10 | 7 - 0 | 8 - 11 | 10 - 11 | 12 - 7 |
| | Hem-Fir | SS | 9 - 9 | 12 - 10 | 16 - 5 | 19 - 11 | 9 - 9 | 12 - 10 | 16 - 5 | 19 - 11 |
| | Hem-Fir | #1 | 9 - 6 | 12 - 7 | 16 - 0 | 18 - 10 | 9 - 6 | 12 - 2 | 14 - 10 | 17 - 2 |
| | Hem-Fir | #2 | 9 - 1 | 12 - 0 | 15 - 2 | 17 - 7 | 8 - 11 | 11 - 4 | 13 - 10 | 16 - 1 |
| | Hem-Fir | #3 | 7 - 6 | 9 - 6 | 11 - 8 | 13 - 6 | 6 - 10 | 8 - 8 | 10 - 7 | 12 - 4 |
| | Southern Pine | SS | 10 - 2 | 13 - 4 | 17 - 0 | 20 - 9 | 10 - 2 | 13 - 4 | 17 - 0 | 20 - 9 |
| | Southern Pine | #1 | 9 - 11 | 13 - 1 | 16 - 9 | 20 - 4 | 9 - 11 | 13 - 1 | 16 - 4 | 19 - 6 |
| | Southern Pine | #2 | 9 - 9 | 12 - 10 | 16 - 1 | 18 - 10 | 9 - 6 | 12 - 4 | 14 - 8 | 17 - 2 |
| | Southern Pine | #3 | 8 - 1 | 10 - 3 | 12 - 2 | 14 - 6 | 7 - 4 | 9 - 5 | 11 - 1 | 13 - 2 |
| | Spruce-Pine Fir | SS | 9 - 6 | 12 - 7 | 16 - 0 | 19 - 6 | 9 - 6 | 12 - 7 | 16 - 0 | 19 - 6 |
| | Spruce-Pine Fir | #1 | 9 - 4 | 12 - 3 | 15 - 5 | 17 - 10 | 9 - 1 | 11 - 6 | 14 - 1 | 16 - 3 |
| | Spruce-Pine Fir | #2 | 9 - 4 | 12 - 3 | 15 - 5 | 17 - 10 | 9 - 1 | 11 - 6 | 14 - 1 | 16 - 3 |
| | Spruce-Pine Fir | #3 | 7 - 6 | 9 - 6 | 11 - 8 | 13 - 6 | 6 - 10 | 8 - 8 | 10 - 7 | 12 - 4 |
| 19.2 in. | Douglas Fir-Larch | SS | 9 - 8 | 12 - 10 | 16 - 4 | 19 - 10 | 9 - 8 | 12 - 10 | 16 - 4 | 19 - 6 |
| | Douglas Fir-Larch | #1 | 9 - 4 | 12 - 4 | 15 - 0 | 17 - 5 | 8 - 10 | 11 - 3 | 13 - 8 | 15 - 11 |
| | Douglas Fir-Larch | #2 | 9 - 2 | 11 - 8 | 14 - 3 | 16 - 6 | 8 - 5 | 10 - 8 | 13 - 0 | 15 - 1 |
| | Douglas Fir-Larch | #3 | 7 - 0 | 8 - 11 | 10 - 11 | 12 - 7 | 6 - 5 | 8 - 2 | 9 - 11 | 11 - 6 |
| | Hem-Fir | SS | 9 - 2 | 12 - 1 | 15 - 5 | 18 - 9 | 9 - 2 | 12 - 1 | 15 - 5 | 18 - 9 |
| | Hem-Fir | #1 | 9 - 0 | 11 - 10 | 14 - 10 | 17 - 2 | 8 - 9 | 11 - 1 | 13 - 6 | 15 - 8 |
| | Hem-Fir | #2 | 8 - 7 | 11 - 3 | 13 - 10 | 16 - 1 | 8 - 2 | 10 - 4 | 12 - 8 | 14 - 8 |
| | Hem-Fir | #3 | 6 - 10 | 8 - 8 | 10 - 7 | 12 - 4 | 6 - 3 | 7 - 11 | 9 - 8 | 11 - 3 |
| | Southern Pine | SS | 9 - 6 | 12 - 7 | 16 - 0 | 19 - 6 | 9 - 6 | 12 - 7 | 16 - 0 | 19 - 6 |
| | Southern Pine | #1 | 9 - 4 | 12 - 4 | 15 - 9 | 19 - 2 | 9 - 4 | 12 - 4 | 14 - 11 | 17 - 9 |
| | Southern Pine | #2 | 9 - 2 | 12 - 1 | 14 - 8 | 17 - 2 | 8 - 8 | 11 - 3 | 13 - 5 | 15 - 8 |
| | Southern Pine | #3 | 7 - 4 | 9 - 5 | 11 - 1 | 13 - 2 | 6 - 9 | 8 - 7 | 10 - 1 | 12 - 1 |
| | Spruce-Pine Fir | SS | 9 - 0 | 11 - 10 | 15 - 1 | 18 - 4 | 9 - 0 | 11 - 10 | 15 - 1 | 17 - 9 |
| | Spruce-Pine Fir | #1 | 8 - 9 | 11 - 6 | 14 - 1 | 16 - 3 | 8 - 3 | 10 - 6 | 12 - 10 | 14 - 10 |
| | Spruce-Pine Fir | #2 | 8 - 9 | 11 - 6 | 14 - 1 | 16 - 3 | 8 - 3 | 10 - 6 | 12 - 10 | 14 - 10 |
| | Spruce-Pine Fir | #3 | 6 - 10 | 8 - 8 | 10 - 7 | 12 - 4 | 6 - 3 | 7 - 11 | 9 - 8 | 11 - 3 |
| 24 in. | Douglas Fir-Larch | SS | 9 - 0 | 11 - 11 | 15 - 2 | 18 - 5 | 9 - 0 | 11 - 11 | 15 - 0 | 17 - 5 |
| | Douglas Fir-Larch | #1 | 8 - 8 | 11 - 0 | 13 - 5 | 15 - 7 | 7 - 11 | 10 - 0 | 12 - 3 | 14 - 3 |
| | Douglas Fir-Larch | #2 | 8 - 3 | 10 - 5 | 12 - 9 | 14 - 9 | 7 - 6 | 9 - 6 | 11 - 8 | 13 - 6 |
| | Douglas Fir-Larch | #3 | 6 - 3 | 8 - 0 | 9 - 9 | 11 - 3 | 5 - 9 | 7 - 3 | 8 - 11 | 10 - 4 |
| | Hem-Fir | SS | 8 - 6 | 11 - 3 | 14 - 4 | 17 - 5 | 8 - 6 | 11 - 3 | 14 - 4 | 16 - 10 |
| | Hem-Fir | #1 | 8 - 4 | 10 - 10 | 13 - 3 | 15 - 5 | 7 - 10 | 9 - 11 | 12 - 1 | 14 - 0 |
| | Hem-Fir | #2 | 7 - 11 | 10 - 2 | 12 - 5 | 14 - 4 | 7 - 4 | 9 - 3 | 11 - 4 | 13 - 1 |
| | Hem-Fir | #3 | 6 - 2 | 7 - 9 | 9 - 6 | 11 - 0 | 5 - 7 | 7 - 1 | 8 - 8 | 10 - 1 |
| | Southern Pine | SS | 8 - 10 | 11 - 8 | 14 - 11 | 18 - 1 | 8 - 10 | 11 - 8 | 14 - 11 | 18 - 1 |
| | Southern Pine | #1 | 8 - 8 | 11 - 5 | 14 - 7 | 17 - 5 | 8 - 8 | 11 - 3 | 13 - 4 | 15 - 11 |
| | Southern Pine | #2 | 8 - 6 | 11 - 0 | 13 - 1 | 15 - 5 | 7 - 9 | 10 - 0 | 12 - 0 | 14 - 0 |
| | Southern Pine | #3 | 6 - 7 | 8 - 5 | 9 - 11 | 11 - 10 | 6 - 0 | 7 - 8 | 9 - 1 | 10 - 9 |
| | Spruce-Pine Fir | SS | 8 - 4 | 11 - 0 | 14 - 0 | 17 - 0 | 8 - 4 | 11 - 0 | 13 - 8 | 15 - 11 |
| | Spruce-Pine Fir | #1 | 8 - 1 | 10 - 3 | 12 - 7 | 14 - 7 | 7 - 5 | 9 - 5 | 11 - 6 | 13 - 4 |
| | Spruce-Pine Fir | #2 | 8 - 1 | 10 - 3 | 12 - 7 | 14 - 7 | 7 - 5 | 9 - 5 | 11 - 6 | 13 - 4 |
| | Spruce-Pine Fir | #3 | 6 - 2 | 7 - 9 | 9 - 6 | 11 - 0 | 5 - 7 | 7 - 1 | 8 - 8 | 10 - 1 |
| | Check sources for availability of lumber in lengths greater than 20 feet. | | | | | | | | | |

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***FOR WOOD CONSTRUCTION, ANSI/AF&PA NDS-2001***
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**Page** **Revision**

27 Replace Table 4.3.8 with the following:

**Table 4.3.8 Incising Factors, $C_i$**

| Design Value | $C_i$ |
|---|---|
| E | 0.95 |
| $F_b$, $F_t$, $F_c$, $F_v$ | 0.80 |
| ~~$F_v$~~, $F_{c\perp}$ | 1.00 |

{Note: Move $F_v$ to the second row}

33 In section 5.2.2, change $F_v$ to $F'_v$ in both equations as follows:

$F'_{rt} = (1/3)F'_v$ for Southern Pine
$F'_{rt} = (1/3)F'_v$ for Douglas Fir-Larch, Douglas…

97 In Footnote 4 of Table 11N, replace "lenth" with "length."

107 In Table 12.2B, first column, shear plate diameter 2-5-8 should be 2-5/8.

117 In Table 13.2.3, metal side plate thickness range for $C_{st} = 0.9$ should be:

3/16" ≤ $t_s$ < 1/4"
{Note: Add ≤ in front of $t_s$; change ≤ to < in front of ¼"}

**2003 ERRATA**
**to the**

***ASD WOOD I-JOIST GUIDELINE***
***included in the Allowable Stress Design (ASD) Manual for Engineered Wood Construction***
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**Page** **Revision**

IJ-29 In Table 6.1, for Detail WIJ-1.5, Min. flange area: 2.25 sq. in.

*{Note: replaces 5.25 sq. in.}*

## 2003 ERRATA
## to the

## *ASD WOOD STRUCTURAL PANELS SUPPLEMENT*
## *included in the Allowable Stress Design (ASD) Manual for Engineered Wood Construction*
(printed version dated 06-02 20M)

**Page** **Revision**

SP-22 Replace Table 4.5 with the following:

**Table 4.5 Panel Size Factor, $C_s$**

| Panel Width, w | $C_s$ |
|---|---|
| w ≤ 8 inches | 0.5 |
| 8 inches < w < 24 inches | $\frac{(8+w)}{32}$ |
| w ≥ 24 inches | 1.0 |

## 2003 ERRATA
## to the

## *ASD STRUCTURAL COMPOSITE LUMBER GUIDELINE*
## *included in the Allowable Stress Design (ASD) Manual for Engineered Wood Construction*
(printed version dated 06-02 20M)

**Page** **Revision**

SCL-9 In Section 4.2.5, revise the following:

$F'_{c\perp} = F_{c\perp} C_M C_t C_{pt} \underline{C_b}$ {Note: Add $C_b$ factor}

$\underline{C_b}$ = per Section 8.3.10 of *NDS*

## 2003 ERRATA/ADDENDUM
## to the
## 2001 Edition of

## *SUPPLEMENT TO THE NDS - DESIGN VALUES FOR WOOD CONSTRUCTION*
## *included in the Allowable Stress Design (ASD) Manual for Engineered Wood Construction*
(printed version dated 06-02 20M)

**Page** **Revision**

4 In Table 2.1, add the following:

| Species or Species Combination | Species That May Be Included in Combination | Grading Rules Agencies | Design Values Provided in Tables |
|---|---|---|---|
| Baldcypress | Baldcypress | SPIB | 4A, 4D |

12 In Section 3.1.3 under the density formula, replace “Table 8A” with “Table 11.3.2A.”

***NDS Supplement changes continued.***

**Page** **Revision**

22 In Table 1D, replace 3-1/8 in. width values with the following:

| Depth | Area | X-X Axis | | | Y-Y Axis | |
|---|---|---|---|---|---|---|
| d (in.) | A (in.$^2$) | $I_x$ (in.$^4$) | $S_x$ (in.$^3$) | $r_x$ (in.) | $I_y$ (in.$^4$) | $S_y$ (in.$^3$) |
| 3-1/8 in. Width ($r_y$ = 0.902 in.) | | | | | | |
| 5 1/2 | 17.19 | ~~17.19~~ 43.33 | ~~43.33~~ 15.76 | ~~15.76~~ 1.588 | 13.99 | 8.952 |
| 6 7/8 | 21.48 | ~~21.48~~ 84.62 | ~~84.62~~ 24.62 | ~~24.62~~ 1.985 | 17.48 | 11.19 |
| 8 1/4 | 25.78 | ~~25.78~~ 146.2 | ~~146.2~~ 35.45 | ~~35.45~~ 2.382 | 20.98 | 13.43 |
| 9 5/8 | 30.08 | ~~30.08~~ 232.2 | ~~232.2~~ 48.25 | ~~48.25~~ 2.778 | 24.48 | 15.67 |
| 11 | 34.38 | ~~34.38~~ 346.6 | ~~346.6~~ 63.02 | ~~63.02~~ 3.175 | 27.97 | 17.90 |
| 12 3/8 | 38.67 | ~~38.67~~ 493.5 | ~~493.5~~ 79.76 | ~~79.76~~ 3.572 | 31.47 | 20.14 |
| 13 3/4 | 42.97 | ~~42.97~~ 677.0 | ~~677.0~~ 98.47 | ~~98.47~~ 3.969 | 34.97 | 22.38 |
| 15 1/8 | 47.27 | ~~47.27~~ 901.1 | ~~901.1~~ 119.1 | ~~119.1~~ 4.366 | 38.46 | 24.62 |
| 16 1/2 | 51.56 | ~~51.56~~ 1170 | ~~1170~~ 141.8 | ~~141.8~~ 4.763 | 41.96 | 26.86 |
| 17 7/8 | 55.86 | ~~55.86~~ 1487 | ~~1487~~ 166.4 | ~~166.4~~ 5.160 | 45.46 | 29.09 |
| 19 1/4 | 60.16 | ~~60.16~~ 1858 | ~~1858~~ 193.0 | ~~193.0~~ 5.557 | 48.96 | 31.33 |
| 20 5/8 | 64.45 | ~~64.45~~ 2285 | ~~2285~~ 221.6 | ~~221.6~~ 5.954 | 52.45 | 33.57 |
| 22 | 68.75 | ~~68.75~~ 2773 | ~~2773~~ 252.1 | ~~252.1~~ 6.351 | 55.95 | 35.81 |
| 23 3/8 | 73.05 | ~~73.05~~ 3326 | ~~3326~~ 284.6 | ~~284.6~~ 6.748 | 59.45 | 38.05 |

31 In Table 4A, add the following design values for Baldcypress

| Species and commercial grade | Size classification | Design values in pounds per square inch (psi) | | | | | | |
|---|---|---|---|---|---|---|---|---|
| | | Bending $F_b$ | Tension parallel to grain $F_t$ | Shear parallel to grain $F_v$ | Compression perpendicular to grain $F_{c\perp}$ | Compression parallel to grain $F_c$ | Modulus of Elasticity E | Grading Rules Agency |
| **Baldcypress** | | | | | | | | |
| Select Structural | 2" & wider | 1200 | 650 | 160 | 615 | 1200 | 1,400,000 | SPIB |
| No. 1 | | 1000 | 550 | 160 | 615 | 1050 | 1,400,000 | |
| No. 2 | | 825 | 450 | 160 | 615 | 900 | 1,300,000 | |
| No. 3 | | 475 | 250 | 160 | 615 | 525 | 1,200,000 | |
| Stud | 2" & wider | 650 | 350 | 160 | 615 | 575 | 1,200,000 | |
| Construction | 2"-4" wide | 925 | 500 | 160 | 615 | 1100 | 1,200,000 | |
| Standard | | 525 | 275 | 160 | 615 | 925 | 1,100,000 | |
| Utility | | 250 | 125 | 160 | 615 | 600 | 1,000,000 | |

44 In Table 4D, add the following design values for Baldcypress

| Species and commercial grade | Size classification | Design values in pounds per square inch (psi) | | | | | | |
|---|---|---|---|---|---|---|---|---|
| | | Bending $F_b$ | Tension parallel to grain $F_t$ | Shear parallel to grain $F_v$ | Compression perpendicular to grain $F_{c\perp}$ | Compression parallel to grain $F_c$ | Modulus of Elasticity E | Grading Rules Agency |
| **Baldcypress** | | | | | | | | |
| Select Structural | 5"x5" & larger | 1150 | 750 | 200 | 615 | 1050 | 1,300,000 | SPIB |
| No. 1 | | 1000 | 675 | 200 | 615 | 925 | 1,300,000 | |
| No. 2 | | 625 | 425 | 175 | 615 | 600 | 1,000,000 | |

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121 Replace Table 13.2.1C with the table on the following page:

## Table 13.2.1C Nominal Wood Capacity Parallel to Grain, $P_w$, for Timber Rivets

### Rivet Length = 2-1/2" $s_p$ = 1" $s_q$ = 1"

| Member Thickness (inches) | Rivets per row | $P_w$ (lbs.) No. of rows per side | | | | | | | | | |
|---|---|---|---|---|---|---|---|---|---|---|---|
| | | 2 | 4 | 6 | 8 | 10 | 12 | 14 | 16 | 18 | 20 |
| 5 | 2 | 2340 | 5610 | 8750 | 12310 | 16120 | 19500 | 22600 | 25910 | 29380 | 33160 |
| | 4 | 3440 | 7390 | 11100 | 15470 | 19950 | 23830 | 27290 | 30900 | 34920 | 39400 |
| | 6 | 4620 | 9160 | 13460 | 18660 | 23860 | 28320 | 31970 | 35960 | 40830 | 46080 |
| | 8 | 5850 | 10840 | 15980 | 21550 | 27350 | 32280 | 36580 | 40900 | 45890 | 51790 |
| | 10 | 6750 | 12500 | 18160 | 24460 | 30810 | 36610 | 40780 | 45840 | 51260 | 57850 |
| | 12 | 7630 | 13830 | 20310 | 27420 | 34280 | 40030 | 45490 | 50870 | 56690 | 63970 |
| | 14 | 8360 | 15480 | 22190 | 30170 | 37450 | 44150 | 49280 | 55620 | 62580 | 70620 |
| | 16 | 8770 | 17110 | 24450 | 32320 | 40570 | 47660 | 52960 | 60450 | 67870 | 76590 |
| | 18 | 9750 | 18580 | 26630 | 34810 | 43460 | 50890 | 57230 | 64170 | 73010 | 81160 |
| | 20 | 10320 | 20320 | 28530 | 36940 | 45920 | 53610 | 60140 | 68380 | 76480 | 84960 |
| 6.75 | 2 | 2710 | 6490 | 10130 | 14260 | 18660 | 22570 | 26170 | 30000 | 34020 | 38390 |
| | 4 | 3980 | 8550 | 12850 | 17910 | 22580 | 26120 | 29190 | 34220 | 40420 | 45620 |
| | 6 | 5350 | 10600 | 15590 | 20390 | 25510 | 29030 | 32670 | 37760 | 45400 | 52330 |
| | 8 | 6770 | 12550 | 18500 | 22880 | 28260 | 31840 | 35470 | 40500 | 47980 | 54310 |
| | 10 | 7810 | 14480 | 21020 | 25280 | 30980 | 34680 | 38400 | 43540 | 51130 | 59140 |
| | 12 | 8830 | 16020 | 23510 | 27430 | 33360 | 37720 | 40900 | 47070 | 55050 | 63330 |
| | 14 | 9670 | 17920 | 25690 | 29640 | 35930 | 40500 | 43810 | 50240 | 58540 | 67000 |
| | 16 | 10160 | 19810 | 28310 | 31700 | 38300 | 43040 | 46460 | 53110 | 63200 | 72360 |
| | 18 | 11290 | 21510 | 30160 | 33950 | 40490 | 45390 | 49750 | 56870 | 67670 | 75240 |
| | 20 | 11950 | 23530 | 32140 | 35770 | 43070 | 48280 | 52920 | 60500 | 72000 | 80080 |
| 8.5 | 2 | 3070 | 7350 | 10580 | 13060 | 16620 | 19300 | 21990 | 26530 | 33760 | 41900 |
| | 4 | 4510 | 9690 | 12400 | 14710 | 18410 | 21240 | 23720 | 27810 | 34060 | 40180 |
| | 6 | 6060 | 12000 | 14390 | 16700 | 20790 | 23640 | 26610 | 30780 | 37040 | 42750 |
| | 8 | 7670 | 13920 | 16320 | 18720 | 23050 | 25970 | 28960 | 33100 | 39250 | 44510 |
| | 10 | 8850 | 15730 | 18150 | 20680 | 25290 | 28330 | 31420 | 35660 | 41930 | 48600 |
| | 12 | 10010 | 17590 | 19970 | 22430 | 27270 | 30870 | 33520 | 38630 | 45240 | 52180 |
| | 14 | 10960 | 19360 | 21660 | 24250 | 29400 | 33190 | 35960 | 41310 | 48200 | 55320 |
| | 16 | 11510 | 21050 | 23410 | 25950 | 31370 | 35320 | 38200 | 43740 | 52130 | 59860 |
| | 18 | 12790 | 22670 | 24900 | 27810 | 33200 | 37290 | 40960 | 46920 | 55920 | 62350 |
| | 20 | 13540 | 24220 | 26510 | 29310 | 35350 | 39720 | 43640 | 49990 | 59580 | 66480 |
| 10.5 | 2 | 3400 | 7730 | 9830 | 11980 | 15210 | 17650 | 20110 | 24260 | 30870 | 38340 |
| | 4 | 5000 | 9490 | 11460 | 13490 | 16860 | 19460 | 21740 | 25500 | 31230 | 36880 |
| | 6 | 6710 | 11400 | 13250 | 15310 | 19060 | 21690 | 24430 | 28270 | 34030 | 39320 |
| | 8 | 8490 | 13150 | 15020 | 17170 | 21150 | 23850 | 26610 | 30440 | 36110 | 41000 |
| | 10 | 9800 | 14810 | 16700 | 18980 | 23230 | 26040 | 28900 | 32840 | 38630 | 44830 |
| | 12 | 11080 | 16520 | 18360 | 20600 | 25060 | 28400 | 30870 | 35610 | 41730 | 48190 |
| | 14 | 12130 | 18140 | 19910 | 22280 | 27040 | 30560 | 33150 | 38110 | 44500 | 51140 |
| | 16 | 12740 | 19680 | 21520 | 23850 | 28870 | 32550 | 35240 | 40390 | 48170 | 55390 |
| | 18 | 14160 | 21160 | 22900 | 25570 | 30570 | 34390 | 37820 | 43350 | 51710 | 57750 |
| | 20 | 14990 | 22580 | 24380 | 26970 | 32570 | 36640 | 40310 | 46220 | 55140 | 61620 |
| 12.5 and greater | 2 | 3540 | 7610 | 9540 | 11590 | 14710 | 17060 | 19440 | 23450 | 29840 | 37060 |
| | 4 | 5210 | 9300 | 11100 | 13040 | 16300 | 18820 | 21030 | 24670 | 30230 | 35700 |
| | 6 | 6990 | 11140 | 12840 | 14810 | 18440 | 20990 | 23650 | 27370 | 32960 | 38100 |
| | 8 | 8860 | 12840 | 14540 | 16620 | 20470 | 23090 | 25780 | 29490 | 35000 | 39750 |
| | 10 | 10220 | 14440 | 16160 | 18370 | 22490 | 25230 | 28010 | 31830 | 37450 | 43490 |
| | 12 | 11550 | 16090 | 17770 | 19940 | 24270 | 27520 | 29920 | 34530 | 40470 | 46760 |
| | 14 | 12650 | 17650 | 19270 | 21580 | 26190 | 29620 | 32150 | 36970 | 43180 | 49650 |
| | 16 | 13290 | 19140 | 20840 | 23100 | 27970 | 31560 | 34180 | 39190 | 46760 | 53800 |
| | 18 | 14760 | 20570 | 22170 | 24770 | 29630 | 33350 | 36690 | 42080 | 50210 | 56110 |
| | 20 | 15630 | 21940 | 23600 | 26130 | 31570 | 35550 | 39120 | 44880 | 53560 | 59880 |

Note: Member dimension is identified as "b" in Figure 13A for connections with steel side plates on opposite sides. For connections having only one plate, member dimension is twice the thickness of the wood member. Linear interpolation for intermediate values shall be permitted.

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# 2001 Edition of

## *ASD/LRFD Supplement on*
## *Special Design Provisions for Wind and Seismic*
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| **Page** | **Revision** |
|---|---|
| 16 | In Table 4.2A revise column headers A & B as follows: |

Replace "Boundary Nail Spacing (inches)" with "Nail Spacing (in.) at diaphragm boundaries (all cases), at continuous panel edges parallel to load (Cases 3 & 4), and at all panel edges (Cases 5 & 6)"

Change boundary nail spacing of 3 inches to 2-1/2 inches

"Panel Edge Nail Spacing (inches)" with "Nail Spacing (in.) at other panel edges (Cases 1, 2, 3 & 4)"

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## *ASD Supplement on*
## *Wood Structural Panels*
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**Page** **Revision**

SP-12 In Table 3.1 change all column headings as follows:

"Strength Axis[a] Perpendicular to Supports" should be "Stress Applied Parallel to Strength Axis[a]"
"Strength Axis[a] Parallel to Supports" should be "Stress Applied Perpendicular to Strength Axis[a]"

SP-13 In Table 3.1.1 change all column headings as follows:

"Stength Axis[a]" to "Stress Applied"
"Perpendicular to Supports" should be "Parallel to Strength Axis[a]"
"Parallel to Supports" should be "Perpendicular to Strength Axis[a]"

SP-14 In Table 3.2 change all column headings as follows:

"Strength Axis[a] Perpendicular to Supports" should be "Stress Applied Parallel to Strength Axis[a]"
"Strength Axis[a] Parallel to Supports" should be "Stress Applied Perpendicular to Strength Axis[a]"

SP-15 In Table 3.2.1 change all column headings as follows:

"Stength Axis[a]" to "Stress Applied"
"Along the Loading Direction" should be "Parallel to Strength Axis[a]"
"Across the Loading Direction" should be "Perpendicular to Strength Axis[a]"

SP-16 In Table 3.3 change all column headings as follows:

"Strength Axis[b] Perpendicular to Supports" should be "Stress Applied Parallel to Strength Axis[b]"
"Strength Axis[b] Parallel to Supports" should be "Stress Applied Perpendicular to Strength Axis[b]"

SP-16 In Table 3.3.1 change all column headings as follows:

"Stength Axis[a]" to "Stress Applied"
"Perpendicular to Supports" should be "Parallel to Strength Axis[a]"
"Parallel to Supports" should be "Perpendicular to Strength Axis[a]"

SP-17 In Table 3.4 change all column headings as follows:

"Strength Axis[b] Perpendicular to Supports" should be "Stress Applied Parallel to Strength Axis[b]"
"Strength Axis[b] Parallel to Supports" should be "Stress Applied Perpendicular to Strength Axis[b]"

SP-18 In Table 3.4.1 change all column headings as follows:

"Stength Axis[a]" to "Stress Applied"
"Perpendicular to Supports" should be "Parallel to Strength Axis[a]"
"Parallel to Supports" should be "Perpendicular to Strength Axis[a]"

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| **Page** | **Revision** |
|---|---|

16 Equation 3.4-5 should be:

$$V_r' = \frac{2}{3} F_v' b \left[ d - \left( \frac{d - d_n}{d_n} \right) e \right]$$

(*Note: Breadth, b, added to equation*)

17 Equation 3.4-6 should be:

$$V_r' = \left[ \frac{2}{3} F_v' b d_e \right] \left[ \frac{d_e}{d} \right]^2$$

(*Note: $d_e$ replaces $d_n$*)

18 Section 3.5.1, last sentence: change the word "be" to "been."

19 Section 3.7.1.3: change $R_e/d$ to $\ell_e/d$.

28 Section 4.4.1.2 (a) – (e) should be:

| | | |
|---|---|---|
| (a) | $d/b \le 2$ | *(remainder unchanged)* |
| (b) | $2 < d/b \le 4$ | *(remainder unchanged)* |
| (c) | $4 < d/b \le 5$ | *(remainder unchanged)* |
| (d) | $5 < d/b \le 6$ | *(remainder unchanged)* |
| (e) | $6 < d/b \le 7$ | *(remainder unchanged)* |

(*Note: < changed to* ≤)

73 Table 11.3.2 Footnote 2: change $F_{e\perp} = 6100G^{1.45} (D)^{½}$ to $F_{e\perp} = 6100G^{1.45} / (D)^{½}$ *(remainder unchanged)*

74 Table 11.3.2A add Yellow Poplar with Specific Gravity = 0.43

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| Page | Revision |
|---|---|
| IJ-32 | Figure 14 (WIJ-1.3) change IIC rating for Carpet & Pad, without Gypsum Concrete from "62" to "66." |
| IJ-33 | Figure 15 (WIJ-1.4) change Report No: UL R14373 should be R10371-1. |

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**Page** **Revision**

9 In Table 3.2A change the Nominal Uniform Loads for sheathing with long dimension parallel to supports with an actual stud spacing of 24" o.c. as follows:

| Span Rating | Nominal Uniform Load |
|---|---|
| 24/16 | Delete $35^2$ and Replace with $25^2$ |
| 32/16 | Delete $45^2$ and Replace with $40^2$ |
| 40/20 | Delete $75^2$ and Replace with $65^2$ |
| 48/24 | Delete $115^2$ and Replace with $90^2$ |

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| **Page** | **Revision** |
|---|---|
| 28 | Section 4.4.1.2 (a) – (e) should be:<br>(a) $d/b \leq 2$ *(remainder unchanged)*<br>(b) $2 < d/b \leq 4$ *(remainder unchanged)*<br>(c) $4 < d/b \leq 5$ *(remainder unchanged)*<br>(d) $5 < d/b \leq 6$ *(remainder unchanged)*<br>(e) $6 < d/b \leq 7$ *(remainder unchanged)*<br><br>(*Note: < changed to* ≤) |
| 73 | Table 11.3.2 Footnote 2: change $F_{e\perp} = 6100G^{1.45} (D)^{½}$ to $F_{e\perp} = 6100G^{1.45} / (D)^{½}$ *(remainder unchanged)* |
| 74 | Table 11.3.2A add <u>Yellow Poplar</u> with Specific Gravity = <u>0.43</u> |

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| **Page** | **Revision** |
|---|---|
| 16 | Equation 3.4-5 should be: |

$$V_r' = \frac{2}{3} F_v' b \left[ d - \left( \frac{d - d_n}{d_n} \right) e \right]$$

(*Note: Breadth, b, added to equation*)

17 Equation 3.4-6 should be:

$$V_r' = \left[ \frac{2}{3} F_v' b d_e \right] \left[ \frac{d_e}{d} \right]^2$$

(*Note: $d_e$ replaces $d_n$*)

35 Revise section 5.4.1 as follows:

Curved bending members having a varying rectangular cross section (see Figure 5A) and taper cut glued laminated bending members shall be designed in accordance with Reference ~~48~~ 52.

72 Add footnote 1 to $K_D$ term in Table 11.3.1B Reduction Terms, $R_d$, as follows:

1. For threaded fasteners where nominal diameter (see Appendix L) is greater than or equal to 0.25" and root diameter is less than 0.25", $R_d = K_D K_\theta$.

**Page** **Revision**

76 In Table 11.5.1A Edge Distance Requirements for loading parallel to grain should be:

**Table 11.5.1A Edge Distance Requirements**[1,2]

| Direction of Loading | Minimum Edge Distance |
|---|---|
| Parallel to Grain: | |
| when $\ell$ /D = 6 | 1.5D |
| when $\ell$ /D > 6 | 1.5D or ½ the spacing between rows, whichever is greater |

(*Remainder Unchanged*)

95-101 In footnote 2 of Tables 11L-11R, diameter range for bending yield strengths should be revised as follows:

$F_{yb}$ = 80,000 psi for 0.177" < D ≤ 0.236"
$F_{yb}$ = 70,000 psi for 0.236" < D ≤ 0.273"

(*Note: 0.236" replaces 0.244" and diameter range for $F_{yb}$ = 70,000 psi added.*)

95 In Table 11L, design values for 0.242" diameter wood screws should be:

| Side Member Thickness $t_s$ in. | Wood Screw Diameter D in. | Wood Screw Number | G=0.67 Red Oak lbs. | G=0.55 Mixed Maple Southern Pine lbs. | G=0.5 Douglas Fir-Larch lbs. | G=0.49 Douglas Fir-Larch (N) lbs. | G=0.46 Douglas Fir(S) Hem-Fir(N) lbs. | G=0.43 Hem-Fir lbs. | G=0.42 Spruce-Pine-Fir lbs. | G=0.37 Redwood (open grain) lbs. | G=0.36 Eastern Softwoods Spruce-Pine-Fir (S) Western Cedars Western Woods lbs. | G=0.35 Northern Species lbs. |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| 1/2 | 0.242 | 14 | 167 | 133 | 119 | 116 | 109 | 101 | 99 | 87 | 85 | 83 |
| 5/8 | 0.242 | 14 | 184 | 142 | 126 | 123 | 114 | 105 | 102 | 89 | 87 | 84 |
| 3/4 | 0.242 | 14 | 203 | 154 | 135 | 131 | 122 | 111 | 108 | 93 | 91 | 87 |
| 1 | 0.242 | 14 | 213 | 178 | 157 | 152 | 139 | 126 | 122 | 102 | 100 | 95 |
| 1 1/4 | 0.242 | 14 | 212 | 177 | 162 | 159 | 150 | 141 | 138 | 114 | 111 | 106 |
| 1 1/2 | 0.242 | 14 | 212 | 177 | 162 | 159 | 150 | 141 | 138 | 122 | 120 | 117 |
| 1 3/4 | 0.242 | 14 | 212 | 177 | 162 | 159 | 150 | 141 | 138 | 122 | 120 | 117 |

96 In Table 11M, design values for 0.242" diameter wood screws should be:

| Side Member Thickness $t_s$ in. | Wood Screw Diameter D in. | Wood Screw Number | G=0.67 Red Oak lbs. | G=0.55 Mixed Maple Southern Pine lbs. | G=0.5 Douglas Fir-Larch lbs. | G=0.49 Douglas Fir-Larch (N) lbs. | G=0.46 Douglas Fir(S) Hem-Fir(N) lbs. | G=0.43 Hem-Fir lbs. | G=0.42 Spruce-Pine-Fir lbs. | G=0.37 Redwood (open grain) lbs. | G=0.36 Eastern Softwoods Spruce-Pine-Fir (S) Western Cedars Western Woods lbs. | G=0.35 Northern Species lbs. |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| 0.075 | 0.242 | 14 | 203 | 174 | 161 | 158 | 151 | 142 | 139 | 125 | 123 | 119 |
| 0.105 | 0.242 | 14 | 212 | 183 | 169 | 166 | 159 | 150 | 147 | 132 | 130 | 126 |
| 0.120 | 0.242 | 14 | 219 | 188 | 175 | 171 | 164 | 154 | 151 | 136 | 134 | 130 |
| 0.134 | 0.242 | 14 | 225 | 194 | 180 | 177 | 169 | 160 | 156 | 141 | 139 | 135 |
| 0.179 | 0.242 | 14 | 250 | 217 | 202 | 198 | 189 | 179 | 175 | 158 | 156 | 152 |
| 0.239 | 0.242 | 14 | 282 | 240 | 221 | 217 | 206 | 194 | 190 | 170 | 166 | 162 |

97 In Table 11N, design values for 0.177" diameter nails should be:

| Side Member Thickness | Nail Diameter | Common Wire Nail / Box Nail / Sinker Nail | G=0.67 Red Oak | G=0.55 Mixed Maple Southern Pine | G=0.5 Douglas Fir-Larch | G=0.49 Douglas Fir-Larch (N) | G=0.46 Douglas Fir(S) Hem-Fir(N) | G=0.43 Hem-Fir | G=0.42 Spruce-Pine-Fir | G=0.37 Redwood (open grain) | G=0.36 Eastern Softwoods Spruce-Pine-Fir (S) Western Cedars Western Woods | G=0.35 Northern Species |
|---|---|---|---|---|---|---|---|---|---|---|---|---|
| $t_s$ in. | D in. | Pennyweight | lbs. | lbs. | lbs. | lbs. | lbs. | lbs. | lbs. | lbs. | lbs. | lbs. |
| 3/4 | 0.177 | 20d | 200 | 153 | 134 | 130 | 121 | 111 | 107 | 92 | 90 | 87 |
| 1 | 0.177 | 20d | 213 | 178 | 155 | 150 | 138 | 125 | 121 | 102 | 99 | 95 |
| 1 1/4 | 0.177 | 20d | 213 | 178 | 163 | 159 | 151 | 141 | 136 | 113 | 110 | 105 |
| 1 1/2 | 0.177 | 20d | 213 | 178 | 163 | 159 | 151 | 141 | 138 | 123 | 121 | 117 |
| 1 3/4 | 0.177 | 20d | 213 | 178 | 163 | 159 | 151 | 141 | 138 | 123 | 121 | 117 |

117 Equation 13.2-3 should be:

$$Q_w = q_w p^{0.8} C_\Delta$$

(*Note: $C_D$ replaces $C_D$*)

152 In Appendix E.6 Sample Solution of Staggered Bolts, equation for $Z_{RT\text{-}2}'$ should be:

$Z_{RT\text{-}2}' = 2(240 \text{ psi})(3.125")(\underline{4}") = 6{,}000$ lbs.

160 In Table I1, fastener diameter range for 70,000 psi and 80,000 psi bending yield strength should be:

$F_{yb} = 80{,}000$ psi for $0.177" < D \leq 0.236"$
$F_{yb} = 70{,}000$ psi for $0.236" < D \leq 0.273"$

(*Note: 0.236" replaces 0.244".*)

161 In Appendix J.5, equation for $Z_\theta'$ should be:

$$Z_q' = \frac{Z_\parallel' Z_\perp'}{Z_\parallel' \sin^2 q + Z_\perp' \cos^2 q}$$

(*Note: $Z_\perp'$ replaces $Z_\perp$*)

167 Revise footnote 3 as follows:

3. Single lead thread shown. Thread length is at least four times the screw diameter or two-thirds of the screw ~~diameter~~ <u>length</u>, whichever is greater. Screws which are too short to accommodate the minimum thread length, have threads extending as close to the underside of the head as practicable.